

PLAN VIEW

PLAN VIEW REF:

- 1) 7/8" HELIAX
- 2) 1 5/8" HELIAX
- 3) STEP BOLTS

BUILDING CODE DESIGN INFORMATION

1. 2015 INTERNATIONAL BUILDING CODE.
2. SEISMIC ZONE - F. ($S_s = 0.54g$ / $S_1 = 0.18g$)
3. TOWER DESIGNED FOR A 120 MPH WIND SPEED NO ICE.
4. TOWER DESIGNED TO EXPOSURE C; RISK CATEGORY IV; TOPO. CAT 1.
5. TOWER DESIGNED TO TIA/EIA-222-H STANDARD.

BASE REACTIONS:(FACTORED)

TOTAL SHEAR = 16 KIPS
 AXIAL LOAD = 68 KIPS
 UPLIFT / LEG = 105 KIPS
 COMP. / LEG = 119 KIPS
 O.T. MOMENT = 1253 FT-K

MEMBER INFORMATION

SECTION	ELEVATION	FACE SIZE	LEG DIA.	DIAGONALS	GIRTS	# OF BAYS
1	0' - 20'	12'-9"	Ø3"	L 2 1/2" x 3/16"	N/A	3 - X
2	20' - 40'	11'-0"	Ø3"	L 2" x 3/16"	N/A	3 - X
3	40' - 60'	9'-3"	Ø2 1/2"	L 1 1/2" x 1/8"	N/A	4 - X
4	60' - 80'	7'-6"	Ø2 1/4"	L 1 1/2" x 1/8"	N/A	4 - X
5	80' - 100'	5'-9"	Ø2 1/4"	L 1 1/2" x 1/8"	N/A	4 - X
6	100' - 120'	4'-0"	Ø1 1/2"	S.R. Ø7/8"	S.R. Ø3/4"	8 - Z
7	120' - 140'	3'-0"	Ø1 1/4"	S.R. Ø3/4"	S.R. Ø5/8"	8 - Z
8	140' - 150'	3'-0"	Ø1 1/4"	S.R. Ø5/8"	S.R. Ø5/8"	4 - Z

ANTENNA INFORMATION

ELEVATION	ANTENNA	LINE
150'	(1) 10' TYP. OMNI/WHIP ANTENNA(S)	(1) 7/8" HELIAX
148'	(1) 2' STD DISH W/ RADOME	(1) 7/8" HELIAX
145'	(1) 2' STD DISH W/ RADOME	(1) 7/8" HELIAX
130'	(1) 10' TYP. OMNI/WHIP ANTENNA(S)	(1) 7/8" HELIAX
120'	(12) 72" x 12" x 3" TYP. FLAT PANEL ANTENNA(S)	(12) 1 5/8" HELIAX

DESIGN NOTES:

- 1) TOWER LEGS ARE CONSTRUCTED OF SOLID ROUND BAR MATERIAL.
- 2) SOLID ROUND 0.75" AND LARGER ASTM A-572 GRADE : 50 KSI MIN.
- 3) SOLID ROUND 0.625" AND SMALLER IS ASTM A-36 GRADE : 36 KSI MIN.
- 4) ALL ANGLE MATERIAL IS ASTM A-529 : 50 KSI MIN.
- 5) ALL BRACE AND FLANGE BOLTS ARE A325-X
- 6) THIS TOWER IS DESIGNED FOR STEP BOLTS UP ONE LEG FOR CLIMBING WITH SAFETY CLIMB DEVICE.
- 7) (6) Ø1 1/4" x 4'-6" LONG (F1554-GR.105) ANCHOR BOLTS PER LEG.
- 8) THIS TOWER IS DESIGNED FOR A 120 M.P.H. WIND SPEED WITH NO ICE AND A 30 M.P.H. WIND SPEED WITH 2.00" IN ICE IN ACCORDANCE WITH THE TIA/EIA-222-H STANDARD. ICE IS CONSIDERED TO INCREASE IN THICKNESS WITH HEIGHT.
- 9) DEFLECTIONS BASED ON A 60 M.P.H. WIND.
- 10) TOWER DESIGNED TO EXPOSURE C; RISK CATEGORY IV; TOPO. CAT 1.

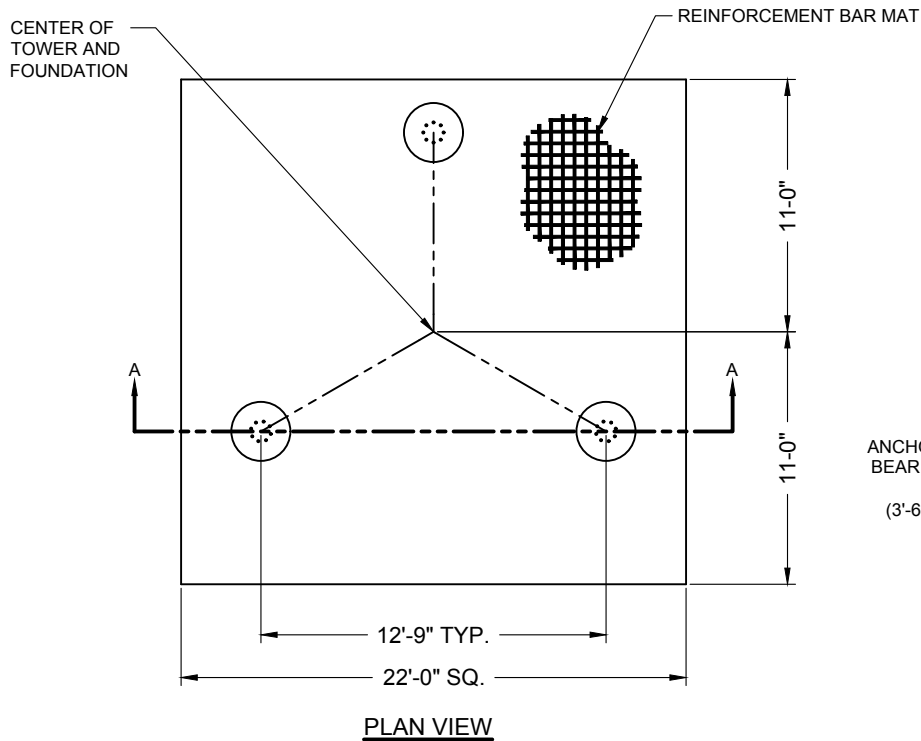
APPROX. WEIGHT
10.09 KIPS

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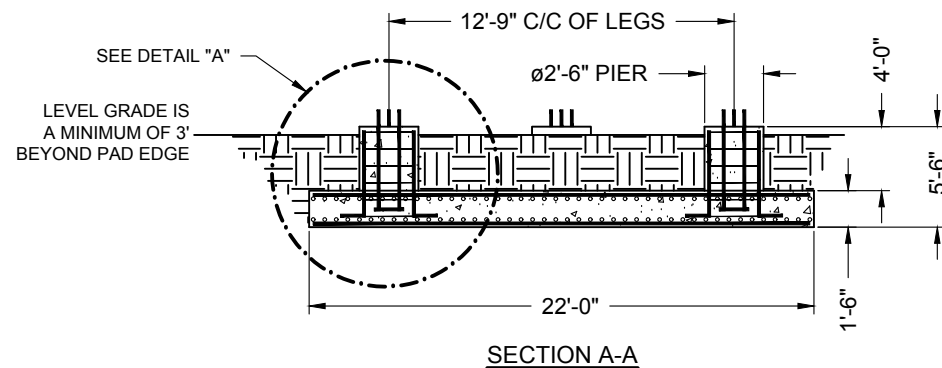
REV #	DESCRIPTION	DATE	BY	UNLESS OTHERWISE NOTED DIMENSIONS ARE IN:	DESCRIPTION:
A	ADDED BUILDING CODE	5/29/2019	JLW	INCHES	TOWER OVERVIEW
				TOLERANCE BANDS:	INDUSTRIAL ENGINEERED SYSTEMS
				.X +3/32"/-0 ANGLES +/- 2"	150' SELF SUPPORT TOWER
				.XX +3/32"/-0	ST. LOUIS, ST. LOUIS CO., MO
				.XXX +1/16"/-0 HOLES +Ø1/16"/-0	
				DRAWN BY: RC	FILE NAME: ER074528A - A
				DATE: 11/12/2018	DESIGN: ER074528
				SCALE: NTS	SHEET A



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TOTAL VOLUME OF CONCRETE = 29.1 YD³

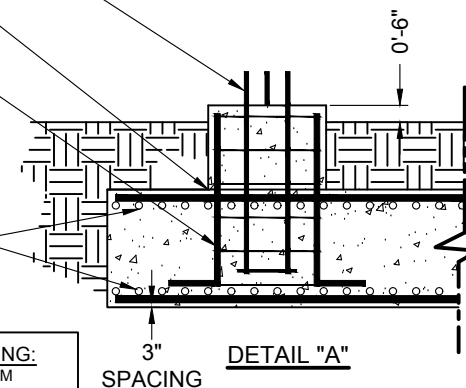


(6) ø1¼" X 4'-6" OVERALL LENGTH ANCHOR BOLTS (F1554-GR105) W/ ¾" THK BEARING PLATE AT THE BOTTOM OF THE ANCHOR BOLT CLUSTER (3'-6" MIN. ANCHOR BOLT EMBEDMENT).

USE EPOXY BONDING AGENT WHEN POURED SEPARATELY

(10) #6 VERTICAL BARS w/ 6" HOOK WITH (5) #4 TIES EQUALLY SPACED

(21) #5 HORIZONTAL BARS x 21'-6" LONG SPACED 12" O.C. EACH WAY AT TOP & BOTTOM OF MAT. (TOTAL=84)



REINFORCEMENT BAR SPLICING:

1. ALL LAP SPLICES SHALL CONFORM TO ACI 318 REQUIREMENTS.
2. REFER TO CHART BELOW WHEN REINFORCEMENT BAR SPLICING IS NECESSARY.

REINFORCING BAR SIZE	LAP SPlice LENGTH
3	15"
4	17"
5	21"
6	26"
7	30"
8	36"
9	46"
10	58"
11	71"

FOUNDATION INSTALLATION/DESIGN NOTES:

1. THIS FOUNDATION IS DESIGNED TO MEET ALL STANDARDS SET FORTH BY ACI 318: AMERICAN CONCRETE INSTITUTE, BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, ANSI/TIA/EIA 222-H: STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES.
2. THIS FOUNDATION IS DESIGNED UTILIZING THE GEOTECHNICAL REPORT PERFORMED BY GEOTECHNOLOGY, INC.; DATED 2-10-2014; GEOTECHNOLOGY PROJECT NO. J021419.01. THE FOUNDATION CONTRACTOR SHALL INSTALL THE FOUNDATIONS IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.
3. ALL WORK PERFORMED FROM THESE DRAWINGS SHOULD BE BY QUALIFIED CONTRACTORS EXPERIENCED IN TOWER FOUNDATION CONSTRUCTION.
4. ALL FOOTING EXCAVATIONS SHALL BE MANUALLY CLEANED PRIOR TO PLACING CONCRETE. COMPACT THE EXPOSED SOIL SURFACE AND ANY GRANULAR FILL UNDER THE FOUNDATION TO 90% OF THE MODIFIED PROCTOR DENSITY.
5. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AFTER 28 DAYS. COPIES OF THE CONCRETE CYLINDER TEST REPORTS SHALL BE SENT TO THE RESIDENT ENGINEER / INSPECTOR.
6. CONCRETE COVER FOR REINFORCING BARS SHALL BE 2" UNLESS NOTED OTHERWISE. ALL REINFORCING BARS SHALL BE GRADE 60 REBAR (MIN YIELD = 60KSI).
7. FIELD BENDING OR WELDING OF REINFORCEMENT BARS IS NOT PERMITTED.
8. PROVIDE CHAMFERS AT ALL EXPOSED CORNERS OF CONCRETE.
9. BACKFILL NEAR AND AROUND THE FOUNDATIONS SHALL BE A WELL GRADED FILL MATERIAL PLACED IN 8" THICK LAYERS THAT HAS BEEN COMPACTED TO 90% OF THE MODIFIED PROCTOR DENSITY PER ASTM D1557.
10. SOME DETAIL HAS BEEN PURPOSELY OMITTED TO CLARIFY ILLUSTRATION.

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REV #	DESCRIPTION	DATE	BY	RC	UNLESS OTHERWISE NOTED DIMENSIONS ARE IN:	DESCRIPTION
A	ADDED DIM TO DETAIL A, AND UPDATED NOTE #6 IN DESIGN NOTES	6/3/2019			INCHES	PAD & PIER FOUNDATION DESIGN
					TOLERANCE BANDS:	INDUSTRIAL ENGINEERED SYSTEMS
					X +3/32" / -0 ANGLES +/- 2"	150' SELF SUPPORT TOWER
					XX +3/32" / -0	ST. LOUIS, ST. LOUIS CO., MO
					.XXX +1/16" / -0 HOLES + Ø1/16" / -0	
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